Deep Mixing Method, The Japanese Experience and Recent Advancement

Advance in Concrete Technology by Hong Kong Concrete Institute

December 6, 2017

Masaki Kitazume, Dr. Eng. Tokyo Institute of Technology

Contents of lecture

- Introduction
 - Classification of admixture technology
- Comparison of concrete and stabilized soil
- Cement Deep Mixing Method
 - laboratory test
 - DM machine
 - Execution
 - Quality control
 - Case history
- Concluding remarks

Classification of admixture stabilization techniques

Place of mixing		Type of mixing	Method	
In-situ	Surface and shallow	mechanical mixing	surface treatment,	
	stabilization		shallow stabilization	
	Mid depth stabilization	mechanical mixing	mid-depth mixing	
	Deep stabilization	mechanical mixing	deep mixing	
		high pressure injection		
		hybrid of above two		
Ex-situ	Mixing during	mixing on belt conveyor	pre-mixing	
	transportation			
		mixing in pipeline	pipe mixing	
	Batch plant mixing	mechanical mixing	pre-mixing	
		mechanical mixing	lightweight geo-	
			material	
		mechanical mixing and	dewatered stabilized	
		high pressure dewatering	soil	







Deep Mixing Method



A deep in-situ soil admixture stabilization technique using cement or lime column diameter: 1 to 1.5 m

column strength: 200 to 2,000 kPa





Deep Mixing Method -historical review of R&D in Japan-

1970 1980 1990 2000 2010 machine development

projects



1968, field trial



1971, first work



1994, Kansai Airport



2010, Haneda Airport

Design standard & manual



1979, lab. test







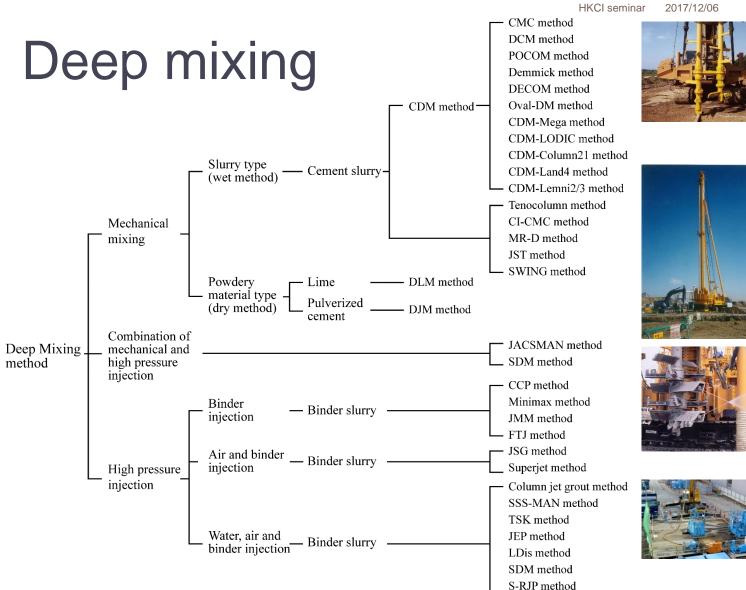




1990, design, lab. test

2002

2007, design 2013













X-jet method





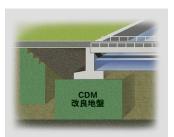


applications

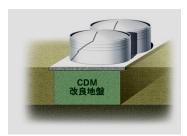
slip failure prevention settlement reduction

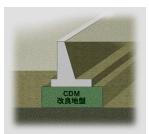


foundation of retaining wall

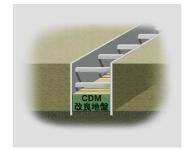


foundation of tank





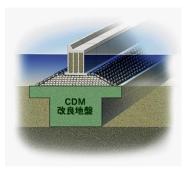
heaving prevention



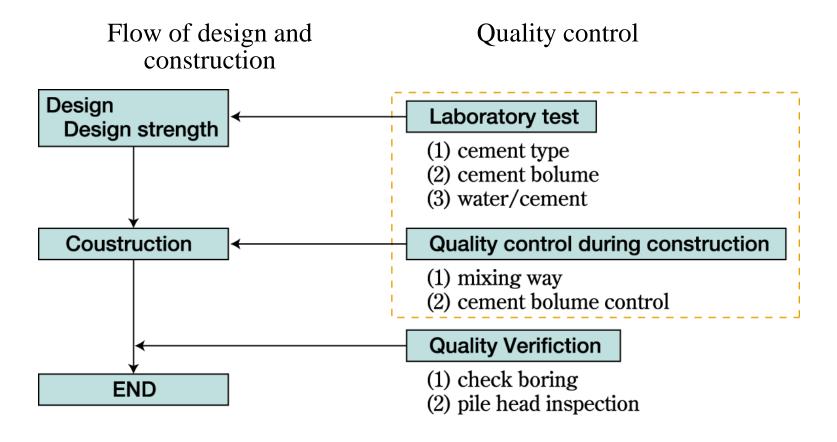
quay wall revetment



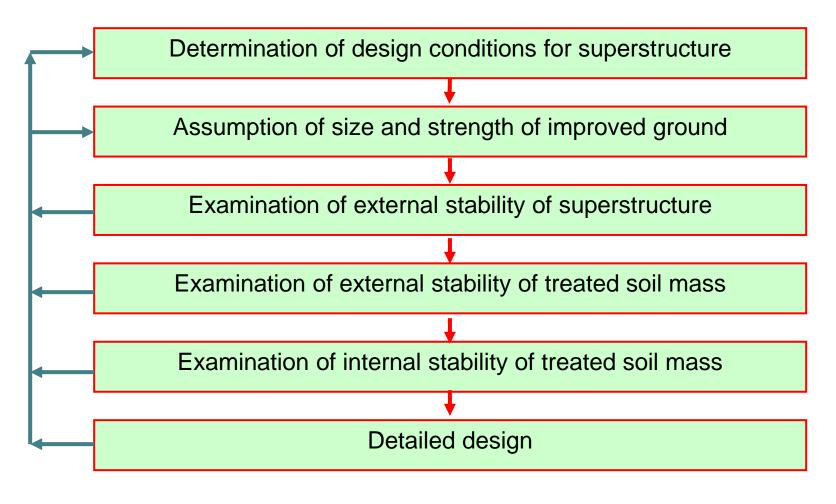
break water



Design and construction procedure



Design procedure for block / wall type improved ground



Laboratory test

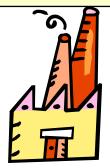






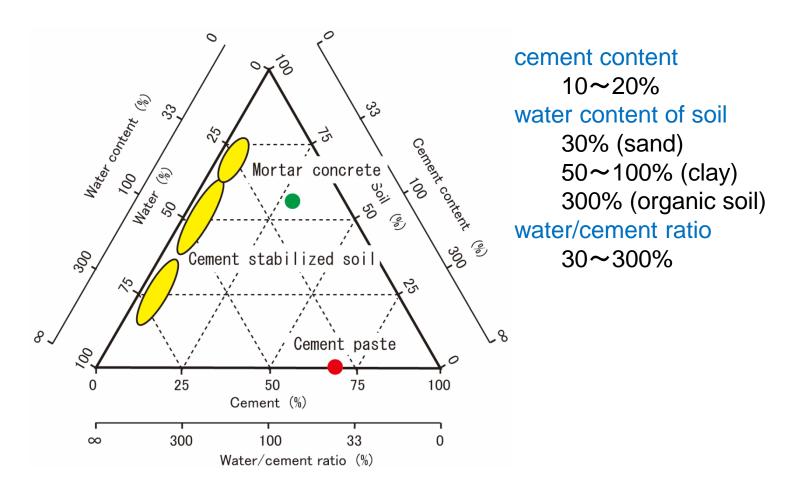
Concrete vs. stabilized soil

Properties	Concrete	Stabilized soil
UCS	18,000 – 45,000 kPa (100,000kPa)	500 – 2,000 kPa
CoV of UCS (lab.)	10 %	10 %
CoV of UCS (field)		25 – 35 %
strength ratio (field/lab.)	1.0	0.3 - 1.0
mixing	cement, water, aggregate, additives	cement, water, soil (sand, clay, organic soil, etc.)
mixing place	in plant and transport to site	field



factory manufacturing order manufacturing

Concrete vs. stabilized soil



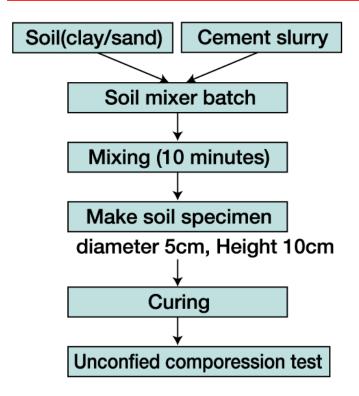
Factors Affecting Strength Increase

1.	Characteristics of binder	Type of binder Quality Mixing water and additives
2.	Characteristics and conditions of soil	Physical, chemical and mineralogical properties of soil Organic content pH of pore water Water content
3.	Mixing conditions	Degree of mixing Timing of mixing/re-mixing Quantity of binder
4.	Curing conditions	Temperature Curing time Humidity Confining pressure Wetting and drying/freezing and thawing, etc.

Laboratory test

OBJECTIVES:

To obtain the mixing condition to achieve the design strength at field.



mixing



molding



capping



trimming



measuring



testing

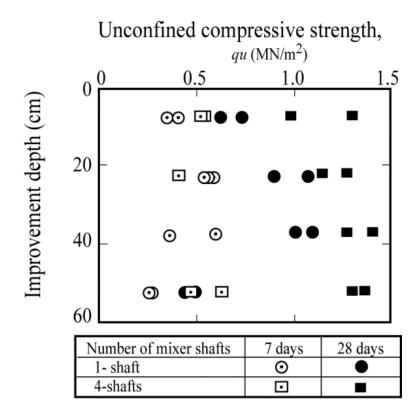


DM machine





Effect of number of mixing shafts



entrained rotation phenomenon a condition in which disturbed soil

adheres to and rotates with the mixing blade without efficient mixing.

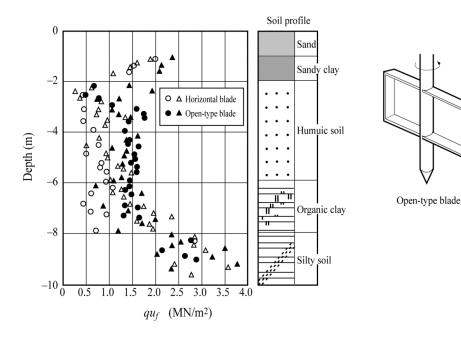
Multi mixing shafts type machine usually has a bracing plate to keep the distance of the two mixing shafts. The plate is also expected to function to increase mixing degree by preventing the "entrained rotation phenomenon".

Effect of shape of mixing blade

effect of free blade

Blade type: B: without Blade type: A: without free blade free blade $qu_l = 1.32 \text{MN/m}^2$ V = 24% Frequency 5 $qu_1 = 0.7 \text{MN/m}^2$ V = 83%1.5 2.0 42.5 0.5 1.0 1.5 2.0 2.5 Unconfined compressive Unconfined compressive strength, strength, $qu (MN/m^2)$ $qu (MN/m^2)$ Free blade

effect of open-type blade





The free blade increases mixing degree by preventing the "entrained rotation phenomenon".

HKCI seminar

2017/12/06

Line up of Cement Deep Mixing Method machines

















Mixing blades

Japan







Nordic countries USA









comparison of wet and dry method - machinery -

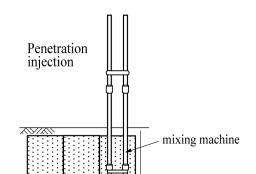
	Cement De	Dry Jet Mixing		
	for marine for on land			
number of mixing shaft	2 - 8	1 - 4	1 - 2	
diameter of mixing blade	1.0 - 1.6 m	1.0 - 1.3 m	0.8 - 1.3 m	
max. depth to be improved	-70 m (below sea level)	-48 m	-33 m	
position of agent outlet	rod and blade	rod and blade	rod	
injection pressure	100-300 kPa	100-300 kPa	700 kPa (air)	

Execution





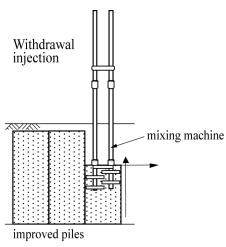
execution procedure

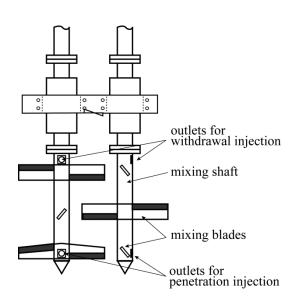


improved piles

penetration injection







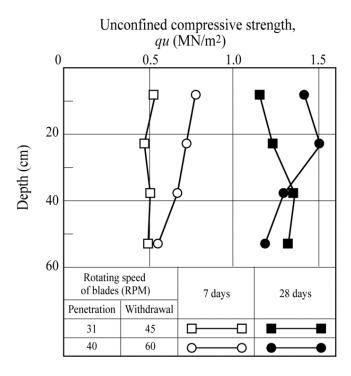
The penetration injection method:

beneficial for the homogeneity of column strength by mixing original soil twice. risk to deadlock or cause serious damage to the machine during penetration.

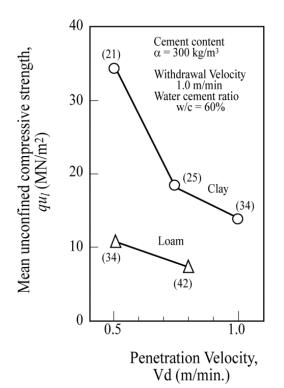
Injection outlet should be installed according to the injection method.

Effect of execution procedure

effect of rotation speed

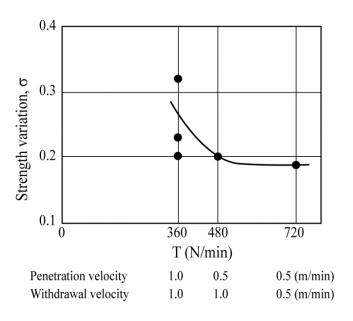


effect of penetration speed



() means coefficient of variation

Blade rotation number



Total of rotations of mixing blades passing through 1 m shaft movement.

$$T = \Sigma M \cdot \left(\frac{N_{\rm d}}{V_{\rm d}} + \frac{N_{\rm u}}{V_{\rm u}}\right)$$

where

T: blade rotation number (N/m)

N_d: number of rotation of mixing blades during penetration (N/min)

N_u: number of rotation of mixing blades during withdrawal (N/min)

V_d: penetration speed of mixing blades (m/min)

V_u: withdrawal speed of mixing blades (m/min)

 Σ M:total number of mixing blades

comparison of wet and dry method

- execution -

	Cement De	Dry lot Mixing		
	for marine	for on land	Dry Jet Mixing	
penetration speed of shaft	1.0 m/min	1.0 m/min	1.0 - 2.0 m/min	
withdrawal speed of shaft	1.0 m/min	0.7 - 1.0 m/min	0.7 - 0.9 m/min	
rotation speed of blade	20 - 60 rpm	20 - 40 rpm	24 - 64 rpm	
blade rotation number	350 /m	350 /m	274 - 284 /m	
amount of stabilizing agent	70 - 300 kg/m ³	70 - 300 kg/m ³	100 - 300 kg/m ³	
injection phase	penetration withdrawal	penetration withdrawal	withdrawal	

Quality control

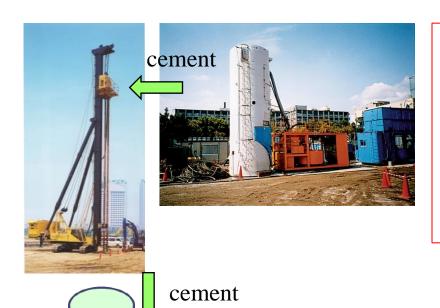








Execution



amount of cement =
 flow rate / min .× travel time

How to control amount of cement to be mixed?

Plant:

supply cement slurry constant at rate DM machine:

keep constant penetration and withdrawal speeds.

requires high capacity to plant and DM machine:

Plant:

manufacturing cement slurry supplying cement slurry to DM machine DM machine:

high power in driving mixing shafts and mixing blades.



blade rotation number

$$T = \sum M \cdot \left(\frac{N_d}{V_d} + \frac{N_u}{V_u} \right)$$

where

T: blade rotation number (n/m)

 ΣM : total number of mixing blades

 N_d : rotation speed of the blades during

penetration (rpm)

 V_d : mixing blade penetration velocity Depth

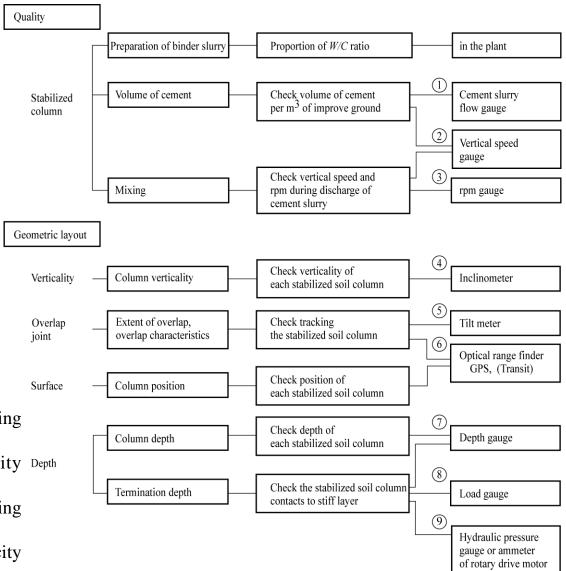
(m/min)

 N_{μ} : rotational speed of the blades during

withdrawal (rpm)

 V_{μ} : mixing blade withdrawal velocity

(m/min)



Quality assurance

Core boring





On land works:

3 cores < 500 columns + 1 core for every 250 columns

Marine works:

3 cores < 500 columns + 1 core for every 500 columns

UC test



The JGS standard of UC test for cohesive soil is applied.

Case history

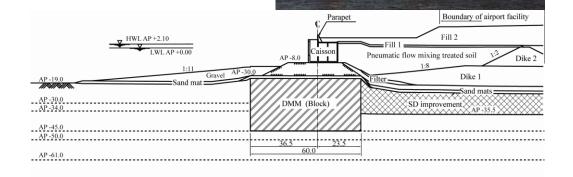




Mixing condition

- Tokyo/Haneda Airport -





	soil property						cement factor		
	depth	<i>w</i> _n (%)	$\rho_{\rm t}$ (g/cm ³)	<i>w</i> ₁ (%)	w _p (%)	Ip	CW rev. (kg/m ³)	CN rev. (kg/m ³)	
surface layer	-19 to -21m	168-177	1.29	-	-	-	165	165	
clay 1-C1	-21 to -30m	132-145	1.34-1.36	132- 137	51-54	78-85	140	145	
clay 1-C2	-30 to -34m	42-117	1.38-1.79	41-118	22-47	19-70	130	135	
clay 2-C	-34 to -45m	35-52	1.75-1.84	32-55	18-24	14-31	110	120	
sand 2-S	-45m deeper	37	1.827	-	-	-			

field strength

					field stre	ngth, $q_{ m uf28}$		bino	der factor
	depth	no. of specimen	ave. (kN/m²)	max. (kN/m²)	min. (kN/m²)	CV (%)	CW rev (kg/m ³)		
surface layer	-19 to -21 m	20	3,409	5,608	2,391	27.1	165	160 ¹⁶⁵ 160	
clay 1-C1	-21 to -30 m	36	4,009	7,981	2,568	28.9	140	120 145 125	
clay 2-C2	-30 to -34 m	16	3,929	6,116	2,257	21.3	130	110 135 120	
sand 2-C	-34 to -45 m	44	4,534	7,595	2,617	26.4	110	80 120 85	
total		116	4,094	7,981	2,257	28.3			

 $> 3,375 \text{ kN/m}^2$

< 35 %

field strength after modification

	depth			field stre	ngth, $q_{ m uf91}$		binder factor		
		no. of specimen	ave. (kN/m²)	max. (kN/m²)	$\min. \\ (kN/m^2)$	CV (%)	CW rev. (kg/m ³)	CN rev. (kg/m ³)	
		30	3,568	6,923	2,027	35.8	160	160	
surface layer	-19 to -21 m	16	4,010	6,052	2,009	31.7	160	160	
clay 1-C1	-21 to -30 m	72	4,410	7,313	2,013	29.8	120	125	
clay 1-C2	-30 to -34 m	32	4,561	7,726	2,092	33.9	110	120	
clay 2-C	-34 to -45 m	88	3,871	6,076	2,038	26.2	80	85	
total		238	4,066	7,313	2,009	31.4			

 $> 3,375 \text{ kN/m}^2$

< 35 %

Concluding remarks

Japanese techniques and experiences on DMM is briefly introduced to show a similar to but quite different technology from concrete technology.

I hope this lecture will promote mutual understanding in concrete engineering and geotechnical engineering.